

## Free Vibration Analysis Of Functionally Graded Rectangular Plate Based On Various Shear Deformation Plate Theory

M. M. Najafizadeh\*, J. Alibabaei shahraki, P. Yousefi, P. Khazaei nezhad

Mechanical Engineering Department of Islamic Azad University, Arak Branch, Arak , Iran.

---

### Abstract

In this paper the free vibration of FGM rectangular plate analyzed by using the Third-order shear deformation theory. By introducing the displacement field according to the Third-order shear deformation plate theory (TSDT), the strain-displacement equations are derived and then by using the Hamilton's principle, dynamic equation for the mentioned plate are achieved and with Navier method whole dynamic equations are converted to an eigen value problem which the natural frequencies of plate can be calculated. Further more, the equations for First-order shear deformation plate theory will be derived and then the results compared with above equation results.

*Keywords:* FGM, Third-order shear deformation plate theory, free vibration, Hamilton's principle.

---

### Nomenclature

$a, b$ length and width of a rectangular plate	$h$ plate thickness
$u, v, w$ displacement in $x, y, z$ direction	$\sigma, \varepsilon$ stress, strain
$\phi_1, \phi_2$ mid-plane rotation	$\rho$ density of plate material
$E, G$ elasticity modulus	$\nu$ poisson's ratio
$N_{ij}$ total in-plane force	$M_{ij}$ total in-plane moment
$A_{ij}$ extensional stiffness	$D_{ij}$ bending stiffness
$B_{ij}$ bending-extensional coupling stiffness	$E_{ij}, F_{ij}, H_{ij}$ high-order stiffness

---

\* Assistant professor, Email: [m-najafizadeh@iau-arak.ac.ir](mailto:m-najafizadeh@iau-arak.ac.ir)

$\omega$ frequency	$\bar{\omega}$ natural frequency
$q$ load	$P_{ij}, R_i$ high-order stress
$p$ material variation profile through the thickness	$T$ temperature
$\alpha$ thermal expansion coefficient	

## 1. Introduction

In recent years functionally graded materials (FGMs) have gained considerable importance as materials to be used in extremely high temperature environments such as nuclear reactors and high-speed spacecraft industries (Yamanouchi et al., [1]). FGMs were first introduced by a group of scientists in Sendai Japan in 1984 (Koizumi, [2]). FGMs are new inhomogeneous materials, in which the mechanical properties vary smoothly and continuously from one surface to the other. This is achieved by gradually varying the volume fraction of the constituent materials. This continuous change in composition results in the graded properties of FGMs (Reddy and Cheng, [3]). This gradation in properties of the material reduces thermal stresses, residual stresses and stress concentration factors (Reddy et al., [4]). Typically these materials are made from a mixture of ceramic and metal or from a combination of different materials. The ceramic constituent of the material provides the high-temperature resistance due to its low thermal conductivity. The ductile metal constituent on the other hand, prevents fracture caused by stresses due to the high temperature gradient in a very short period of time. Furthermore a mixture of ceramic and metal with a continuously varying volume fraction can be easily manufactured (Fukui, [5]).

Studies on vibration of rectangular plates are extensive. Many of these studies are for isotropic and composite plates. In recent years many researchs about rectangular plates such as stability and vibration plates according to a Higher-Order Shear Deformation Theory [6,7], relationship between vibration frequencies of Reddy and

Kirchhoff Plates With Simply Supported Edges [8], free vibrations of laminated composite plates using second-order shear deformation theory and Layerwise theory [9,10], Analysis of laminated composite plates using HSDT [11], theory of plates and shells [12], buckling and vibration of laminated composite plate using various plate theories [13] has been done.

The First-order Shear Deformation Theory (FSDT) is the simplest plate theory that accounts for transverse shear strains which are represented as constant through the plate thickness, and the theory requires shear correction factors to compute transverse shear forces. In the Third-order Shear Deformation Theory (TSDT) of Reddy, the transverse shear stresses are represented as cubic through the thickness and consequently it isn't require to shear correction factors. The theory also contains the First-order Shear Deformation Theory as a special case. Here we develop the equations of motion of functionally graded plates using TSDT.

In the present work, vibration of functionally graded rectangular plate based on the third order shear deformation theory is studied. The objective is to study the frequency characteristics, the influence of the constituent volume fractions, and the affects of the configurations of the constituent materials on the natural frequencies.

## **2. Third-order theory of shear deformation plate**

Consider a plate of total thickness  $h$  and composed of functionally graded material through the thickness. It is assumed that the material is isotropic and the grading is assumed to be only through the thickness. The  $xy$ -plane is taken to be the undeformed midplane of the plate with the  $z$ -axis positive upward from the midplane. Further, we restrict the formulation to linear elastic material behavior, small strains and displacements, and to the case in which the temperature field is known.

## 2.1. Displacement field

The Third-order shear deformation theory of Reddy used in the present study is based on the following displacement field [14]:

$$\begin{cases} u(x, y, z) = u_0(x, y) + z \cdot \phi_1(x, y) + z^2 \psi_1(x, y) + z^3 u_3(x, y) \\ v(x, y, z) = v_0(x, y) + z \cdot \phi_2(x, y) + z^2 \psi_2(x, y) + z^3 v_3(x, y) \\ w(x, y, z) = w_0(x, y) \end{cases} \quad (1)$$

These equations can be reduced by satisfying the stress-free conditions on the top and bottom faces of the plate, which are equivalent to:

$$\begin{cases} u(x, y, z) = u_0(x, y) + z \cdot \phi_1(x, y) - C_1 z^3 \left( \phi_1 + \frac{\partial w_0}{\partial x} \right) \\ v(x, y, z) = v_0(x, y) + z \cdot \phi_2(x, y) - C_1 z^3 \left( \phi_2 + \frac{\partial w_0}{\partial y} \right) \\ w(x, y, z) = w_0(x, y) \end{cases} \quad (2)$$

where  $(u_0, v_0, w_0)$  and  $(\phi_1, \phi_2)$  are displacement and rotation of normal lines on the plane  $z = 0$ , respectively. Also,  $C_1 = 4/(3h^2)$ .

## 2.2. Strain

The linear strain-displacement relations are given by:

$$\begin{Bmatrix} \varepsilon_{11} \\ \varepsilon_{22} \\ \varepsilon_{12} \end{Bmatrix} = \begin{Bmatrix} k_{11}^{(0)} \\ k_{22}^{(0)} \\ k_{12}^{(0)} \end{Bmatrix} + z \begin{Bmatrix} k^{(1)}_{11} \\ k^{(1)}_{22} \\ k^{(1)}_{12} \end{Bmatrix} + z^3 \begin{Bmatrix} k^{(3)}_{11} \\ k^{(3)}_{22} \\ k^{(3)}_{12} \end{Bmatrix} \quad (3)$$

$$\begin{Bmatrix} \varepsilon_{13} \\ \varepsilon_{23} \end{Bmatrix} = \begin{Bmatrix} \gamma_{13}^{(0)} \\ \gamma_{23}^{(0)} \end{Bmatrix} + z^2 \begin{Bmatrix} \gamma_{13}^{(2)} \\ \gamma_{23}^{(2)} \end{Bmatrix} \quad (4)$$

$$\begin{cases} k_{11}^{(0)} \\ k_{22}^{(0)} \\ k_{12}^{(0)} \end{cases} = \begin{cases} \frac{\partial u_0}{\partial x} \\ \frac{\partial v_0}{\partial y} \\ \frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \end{cases}, \quad \begin{cases} k_{11}^{(1)} \\ k_{22}^{(1)} \\ k_{12}^{(1)} \end{cases} = \begin{cases} \frac{\partial \phi_1}{\partial x} \\ \frac{\partial \phi_2}{\partial y} \\ \frac{\partial \phi_1}{\partial y} + \frac{\partial \phi_2}{\partial x} \end{cases} \quad (5)$$

$$\begin{cases} k_{11}^{(3)} \\ k_{22}^{(3)} \\ k_{12}^{(3)} \end{cases} = -C_1 \begin{cases} \left( \frac{\partial \phi_1}{\partial x} + \frac{\partial^2 w_0}{\partial x^2} \right) \\ \left( \frac{\partial \phi_2}{\partial y} + \frac{\partial^2 w_0}{\partial y^2} \right) \\ \left( \frac{\partial \phi_2}{\partial x} + 2 \frac{\partial^2 w_0}{\partial x \partial y} + \frac{\partial \phi_1}{\partial y} \right) \end{cases}$$

$$\begin{cases} \gamma_{13}^{(0)} \\ \gamma_{23}^{(0)} \end{cases} = \begin{cases} \left( \phi_1 + \frac{\partial w_0}{\partial x} \right) \\ \left( \phi_2 + \frac{\partial w_0}{\partial y} \right) \end{cases}, \quad \begin{cases} \gamma_{13}^{(2)} \\ \gamma_{23}^{(2)} \end{cases} = -3C_1 \begin{cases} \left( \phi_1 + \frac{\partial w_0}{\partial x} \right) \\ \left( \phi_2 + \frac{\partial w_0}{\partial y} \right) \end{cases} \quad (6)$$

### 2.3. Functionally Graded Plates

We assume that the material property gradation is only through the thickness. FGM properties such as density  $\rho$ , elasticity modulus  $E$  and  $G$  are functions of volumetric ratio and the components. The poisson's coefficient is considered as constant. If we consider the normal axis of midplane as  $z$ , we'll have:

$$\begin{aligned} E(z) &= (E_c - E_m) \left( \frac{z}{h} + \frac{1}{2} \right)^p + E_m \\ \rho(z) &= (\rho_c - \rho_m) \left( \frac{z}{h} + \frac{1}{2} \right)^p + \rho_m \\ G = G(z) &= (G_c - G_m) \left( \frac{z}{h} + \frac{1}{2} \right)^p + G_m \end{aligned} \quad (8)$$

where  $E_c$ ,  $\rho_c$ ,  $G_c$  denote the property of top face and  $E_m$ ,  $\rho_m$ ,  $G_m$  denote bottom face property,  $h$  is the total thickness of the plate and  $p$  is a parameter that dictates the material variation profile through the thickness.

## 2.4. Stress-Strain Relations

The stress-Strain relations are similar to isotropic plate's relations, but with this difference that modul  $E$  is not constant, but it is according to equation (8).

$$\begin{Bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{23} \\ \sigma_{13} \\ \sigma_{12} \end{Bmatrix} = \begin{bmatrix} Q_{11} & Q_{12} & 0 & 0 & 0 \\ Q_{12} & Q_{22} & 0 & 0 & 0 \\ 0 & 0 & Q_{44} & 0 & 0 \\ 0 & 0 & 0 & Q_{55} & 0 \\ 0 & 0 & 0 & 0 & Q_{66} \end{bmatrix} \cdot \begin{Bmatrix} \varepsilon_{11} \\ \varepsilon_{22} \\ \varepsilon_{23} \\ \varepsilon_{13} \\ \varepsilon_{12} \end{Bmatrix} - \begin{Bmatrix} 1 \\ 1 \\ 0 \\ 0 \\ 0 \end{Bmatrix} \alpha \cdot \Delta T$$

where:

$$Q_{11} = Q_{22} = \frac{E}{1-\nu^2}, \quad Q_{12} = \nu \cdot Q_{11}, \quad Q_{44} = Q_{55} = Q_{66} = \frac{E}{2(1+\nu)} \quad (10)$$

## 2.5. Equations of motion

The equation of motion based on TSDT by using Hamilton's principle are:

$$\frac{\partial N_{11}}{\partial x} + \frac{\partial N_{12}}{\partial y} = -q_1 + I_0 \ddot{u}_0 + (I_1 - C_1 I_3) \ddot{\phi}_1 - C_1 I_3 \frac{\partial \ddot{w}_0}{\partial x} \quad (11)$$

$$\frac{\partial N_{22}}{\partial y} + \frac{\partial N_{12}}{\partial x} = -q_2 + I_0 \ddot{u}_2 + (I_1 - C_1 I_3) \ddot{\phi}_2 - C_1 I_3 \frac{\partial \ddot{w}_0}{\partial y} \quad (12)$$

$$\begin{aligned} & \frac{\partial^2 P_{11}}{\partial x^2} C_1 + \frac{\partial^2 P_{22}}{\partial y^2} C_1 + 2 \frac{\partial^2 P_{12}}{\partial x \partial y} C_1 + \frac{\partial Q_1}{\partial x} - 3C_1 \frac{\partial R_{13}}{\partial x} + \frac{\partial Q_2}{\partial y} - 3C_1 \frac{\partial R_{23}}{\partial y} \\ & = -q_3 + I_0 \ddot{w}_0 - C_1^2 I_6 \left( \frac{\partial^2 \ddot{w}_0}{\partial x^2} + \frac{\partial^2 \ddot{w}_0}{\partial y^2} \right) + C_1 \left[ (I_4 - C_1 I_6) \left( \frac{\partial \ddot{\phi}_1}{\partial x} + \frac{\partial \ddot{\phi}_2}{\partial y} \right) + I_3 \left( \frac{\partial \ddot{u}_0}{\partial x} + \frac{\partial \ddot{v}_0}{\partial y} \right) \right] \end{aligned} \quad (13)$$

$$\begin{aligned} & \frac{\partial M_{11}}{\partial x} - C_1 \frac{\partial P_{11}}{\partial x} + \frac{\partial M_{12}}{\partial y} - C_1 \frac{\partial P_{12}}{\partial y} + 3C_1 R_{13} - Q_1 = (I_1 - C_1 I_3) \ddot{u}_0 \\ & + (I_2 - 2C_1 I_4 + C_1^2 I_6) \ddot{\phi}_1 - C_1 (I_4 - C_1 I_6) \frac{\partial \ddot{u}_3}{\partial x} \end{aligned} \quad (14)$$

$$\begin{aligned} & \frac{\partial M_{22}}{\partial y} - C_1 \frac{\partial P_{22}}{\partial y} + \frac{\partial M_{12}}{\partial x} - C_1 \frac{\partial P_{12}}{\partial x} + 3C_1 R_{23} - Q_2 = (I_1 - C_1 I_3) \ddot{v}_0 \\ & + (I_2 - 2C_1 I_4 + C_1^2 I_6) \ddot{\phi}_2 - C_1 (I_4 - C_1 I_6) \frac{\partial \ddot{w}_0}{\partial y} \end{aligned} \quad (15)$$

where:

$$\begin{Bmatrix} N_{11} \\ N_{22} \\ N_{12} \end{Bmatrix} = \int_{-h/2}^{h/2} \begin{Bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{Bmatrix} dz, \quad \begin{Bmatrix} M_{11} \\ M_{22} \\ M_{12} \end{Bmatrix} = \int_{-h/2}^{h/2} \begin{Bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{Bmatrix} z dz \quad (16)$$

$$\begin{Bmatrix} Q_1 \\ Q_2 \end{Bmatrix} = \int_{-h/2}^{h/2} \begin{Bmatrix} \sigma_{13} \\ \sigma_{23} \end{Bmatrix} dz \quad (17)$$

$N_{ij}$  represent the total in-plane force resultant and  $M_{ij}$  the total moment resultants, and

$$\begin{Bmatrix} P_{11} \\ P_{22} \\ P_{12} \end{Bmatrix} = \int_{-h/2}^{h/2} \begin{Bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{12} \end{Bmatrix} z^3 dz, \quad \begin{Bmatrix} R_1 \\ R_2 \end{Bmatrix} = \int_{-h/2}^{h/2} \begin{Bmatrix} \sigma_{13} \\ \sigma_{23} \end{Bmatrix} z^2 dz \quad (18)$$

which  $P_{ij}$  and  $R_i$  show the high-order stress resultants. Also,

$$I_i = \int_{-h/2}^{h/2} \rho(z)^i dz \quad (i = 0, 1, \dots, 6) \quad (19)$$

Consider the following description:

$$(A_{ij}, B_{ij}, D_{ij}, E_{ij}, F_{ij}, G_{ij}, H_{ij}) = \int_{-h/2}^{h/2} Q_{ij}(1, z, z^2, z^3, z^4, z^5, z^6) dz \quad (20)$$

where  $Q_{ij}$  comes from (10) and  $A_{ij}$  denote extensional stiffness,  $D_{ij}$  the bending stiffness,  $B_{ij}$  the bending-extensional coupling stiffnesses, and  $E_{ij}$ ,  $F_{ij}$  and  $H_{ij}$  are high-order stiffness.

Parameters  $A_{ij}$ ,  $D_{ij}$  are described for  $i, j = 1, 2, 6$ ,  $F_{ij}$  for  $i, j = 4, 5$  and  $B_{ij}$ ,  $E_{ij}$ ,  $H_{ij}$  for  $i, j = 1, 2, 6$ . It is important to see that  $E_{ij}$ ,  $F_{ij}$  and  $H_{ij}$  involve fourth or higher powers of the thickness, so they are expected to affect little to thin and homogeneous plates, Therefore the stress resultants are related to the strain by the relations as following:

$$\begin{Bmatrix} \{N\} \\ \{M\} \\ \{P\} \end{Bmatrix} = \begin{bmatrix} [A] & [B] & [E] \\ [B] & [D] & [F] \\ [E] & [F] & [H] \end{bmatrix} \begin{Bmatrix} \{k^{(0)}\} \\ \{k^{(1)}\} \\ \{k^{(3)}\} \end{Bmatrix} \quad (21)$$

$$\begin{Bmatrix} \{Q\} \\ \{R\} \end{Bmatrix} = \begin{bmatrix} [A] & [D] \\ [D] & [F] \end{bmatrix} \begin{Bmatrix} \{\gamma^{(0)}\} \\ \{\gamma^{(2)}\} \end{Bmatrix} \quad (22)$$

## 2.6. Solutions for a simply supported plate

### 2.6.1. Boundary conditions

The simply supported boundary conditions for the Third-order shear deformation plate theory are:

$$\begin{aligned}
 u_1(x,0,t) = 0 \quad , \quad \phi_x(x,0,t) = 0 \quad , \quad u_1(x,b,t) = 0 \quad , \quad \phi_x(x,b,t) = 0 \\
 u_2(0,y,t) = 0 \quad , \quad \phi_y(0,y,t) = 0 \quad , \quad u_2(a,y,t) = 0 \quad , \quad \phi_y(a,y,t) = 0 \\
 u_3(x,0,t) = 0 \quad , \quad u_3(x,b,t) = 0 \quad , \quad u_3(0,y,t) = 0 \quad , \quad u_3(a,y,t) = 0 \\
 N_{11}(0,y,t) = 0 \quad , \quad N_{11}(a,y,t) = 0 \quad , \quad N_{22}(x,0,t) = 0 \quad , \quad N_{11}(x,b,t) = 0 \\
 \bar{M}_{11}(0,y,t) = 0 \quad , \quad \bar{M}_{11}(a,y,t) = 0 \quad , \quad \bar{M}_{22}(x,0,t) = 0 \quad , \quad \bar{M}_{11}(x,b,t) = 0
 \end{aligned} \tag{23}$$

where

$$\bar{M}_{\alpha\beta} = M_{\alpha\beta} - C_1 P_{\alpha\beta} \tag{24}$$

### 2.6.2. The Navier Solutions

The five equations of motion (11)-(15), Solved by Navier solutions for simply supported plates. The boundary conditions in equation (23) are satisfied by the following expansions:

$$u_0(x,y,t) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} U_{mn}(t) \cos \alpha x \sin \beta y \quad , \quad U_{mn}(t) = U e^{-i\omega t} \tag{25a}$$

$$v_0(x,y,t) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} V_{mn}(t) \sin \alpha x \cos \beta y \quad , \quad V_{mn}(t) = V e^{-i\omega t} \tag{25b}$$

$$w_0(x,y,t) = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} w_{mn}(t) \sin \alpha x \sin \beta y \quad , \quad w_{mn}(t) = W e^{-i\omega t} \tag{25c}$$

$$\phi_1(x,y,t) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn}(t) \cos \alpha x \sin \beta y \quad , \quad A_{mn}(t) = A e^{-i\omega t} \tag{25d}$$

$$\phi_2(x,y,t) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} B_{mn}(t) \sin \alpha x \cos \beta y \quad , \quad B_{mn}(t) = B e^{-i\omega t} \tag{25e}$$

where  $\alpha = m\pi/a$  ,  $\beta = n\pi/b$  and  $\omega$  is natural frequency. Assume  $q_1 = q_2 = 0$  , the normal load  $q_3$  can be expanded in double Fourier sin series:



$$q(x, y, t) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} q_{mn}(t) \cos \alpha x \sin \beta y \quad (26)$$

$$q_{mn}(z, t) = \frac{4}{ab} \int_a^b \int_a^b q(x, y, t) \sin \alpha x \sin \beta y \, dx \, dy \quad (27)$$

## 2.7. Natural frequency equation of simply supported FGM rectangular plate (TSDT)

Substitution of equations (25a)-(25e) into equations (11)-(15) and simplifying the resultants relations, then we obtain the following equation:

$$[C] \begin{Bmatrix} U_{mn} \\ V_{mn} \\ W_{mn} \\ A_{mn} \\ B_{mn} \end{Bmatrix} + [M] \begin{Bmatrix} \ddot{U}_{mn} \\ \ddot{V}_{mn} \\ \ddot{W}_{mn} \\ \ddot{A}_{mn} \\ \ddot{B}_{mn} \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ q_{mn} \\ 0 \\ 0 \end{Bmatrix} \quad (28)$$

or

$$[C] \begin{Bmatrix} U \\ V \\ W \\ A \\ B \end{Bmatrix} - \omega^2 [M] \begin{Bmatrix} U \\ V \\ W \\ A \\ B \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ q \\ 0 \\ 0 \end{Bmatrix} \quad (29)$$

where:

$$C = \begin{bmatrix} c_{11} & c_{12} & c_{13} & c_{14} & c_{15} \\ c_{21} & c_{22} & c_{23} & c_{24} & c_{25} \\ c_{31} & c_{32} & c_{33} & c_{34} & c_{35} \\ c_{41} & c_{42} & c_{43} & c_{44} & c_{45} \\ c_{51} & c_{52} & c_{53} & c_{54} & c_{55} \end{bmatrix}, \quad M = \begin{bmatrix} m_{11} & m_{12} & m_{13} & m_{14} & m_{15} \\ m_{21} & m_{22} & m_{23} & m_{24} & m_{25} \\ m_{31} & m_{32} & m_{33} & m_{34} & m_{35} \\ m_{41} & m_{42} & m_{43} & m_{44} & m_{45} \\ m_{51} & m_{52} & m_{53} & m_{54} & m_{55} \end{bmatrix} \quad (30)$$

The components of the matrix  $C$  are defined by:

$$\begin{aligned} c_{11} &= A_{11}\alpha^2 + A_{66}\beta^2 \\ c_{12} &= (A_{12} + A_{66})\alpha\beta \\ c_{13} &= -C_1[E_{11}\alpha^2 + (E_{12} + 2E_{66})\beta^2]\alpha \\ c_{14} &= (B_{11} - C_1E_{11})\alpha^2 + (B_{66} - C_1E_{66})\beta^2 \\ c_{15} &= [(B_{12} - C_1E_{12}) + (B_{66} - C_1E_{66})]\alpha\beta \\ c_{22} &= A_{66}\alpha^2 + A_{22}\beta^2 \\ c_{23} &= -C_1[E_{22}\beta^2 + (E_{12} + 2E_{66})\alpha^2]\beta \\ c_{24} &= c_{15} \end{aligned}$$

$$\begin{aligned}
c_{25} &= (B_{66} - C_1 E_{66})\alpha^2 + (B_{22} - C_1 E_{22})\beta^2 \\
c_{33} &= (A_{55} - 2C_1 D_{55} + C_1^2 F_{55})\alpha^2 + (A_{44} - 2C_1 D_{44} + C_1^2 F_{44})\beta^2 + C_1^2 [H_{11}\alpha^4 + 2(H_{12} + 2H_{66})\alpha^2\beta^2 + H_{22}\beta^4] \\
c_{34} &= (A_{55} - 2C_1 D_{55} + C_1^2 F_{55})\alpha - C_1 [(F_{11} - C_1 H_{11})\alpha^3 + ((F_{12} - C_1 H_{12}) + 2(F_{66} - C_1 H_{66}))\alpha\beta^2] \\
c_{35} &= (A_{44} - 2C_1 D_{44} + C_1^2 F_{44})\beta - C_1 [(F_{22} - C_1 H_{22})\beta^3 + ((F_{12} - C_1 H_{12}) + 2(F_{66} - C_1 H_{66}))\alpha^2\beta] \\
c_{44} &= (A_{55} - 2C_1 D_{55} + C_1^2 F_{55}) + (D_{11} - 2C_1 F_{11} + C_1^2 H_{11})\alpha^2 + (D_{66} - 2C_1 F_{66} + C_1^2 H_{66})\beta^2 \\
c_{45} &= [(D_{12} - 2C_1 F_{12} + C_1^2 H_{12}) + (D_{66} - 2C_1 F_{66} + C_1^2 H_{66})]\alpha\beta \\
c_{55} &= (A_{44} - 6C_1 D_{44} + 9C_1^2 F_{44}) + (D_{66} - 2C_1 F_{66} + C_1^2 H_{66})\alpha^2 + (D_{22} - 2C_1 F_{22} + C_1^2 H_{22})\beta^2
\end{aligned} \tag{31}$$

And the components of the matrix  $M$  are defined by:

$$\begin{aligned}
m_{11} &= I_0 \\
m_{22} &= I_0 \\
m_{33} &= I_0 + C_1^2 I_6 (\alpha^2 + \beta^2) \\
m_{34} &= -C_1 (I_4 - C_1 I_6) \alpha \\
m_{35} &= -C_1 (I_4 - C_1 I_6) \beta \\
m_{44} &= I_2 - 2C_1 I_4 + C_1^2 I_6 \\
m_{55} &= I_2 - 2C_1 I_4 + C_1^2 I_6
\end{aligned} \tag{32}$$

The equations (28) can be specialized to static response, buckling, and vibrations. To achieve the natural frequency, we set  $q_{mn} = 0$ , therefore equations (29) become as following:

$$[C] \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \\ \phi_1 \\ \phi_2 \end{Bmatrix} - \bar{\omega}^2 [M] \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \\ \phi_1 \\ \phi_2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$

or

$$[c_{ij} - m_{ij} \bar{\omega}^2] \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \\ \phi_1 \\ \phi_2 \end{Bmatrix} = 0 \tag{33}$$

Setting the determinant of matrix  $[c_{ij} - m_{ij}\bar{\omega}^2]$  equal to zero and solving the achieved equation for  $\bar{\omega}$  (frequency), values of natural frequency for simply supported rectangular FGM plate will be derived.

### 3. First-order theory of shear deformation FSDT

Using the achieved equations for FGM rectangular plate in TSDT, we will derive the above equations for a rectangular plate in FSDT. To do this, by setting  $C_1 = 0$  in any equation of the last chapter, new equations will be derived with FSDT. It is important to know by setting  $C_1 = 0$ , the value of shear strain become independent of thickness.

#### 3.1. Displacement field

Setting  $C_1 = 0$  in equation (2), we obtain displacement field for FSDT as following:

$$\begin{cases} u(x, y, z) = u_0(x, y) + z \cdot \phi_1(x, y) \\ v(x, y, z) = v_2(x, y) + z \cdot \phi_2(x, y) \\ w(x, y, z) = w_0(x, y) \end{cases} \quad (34)$$

#### 3.2. Strain

In the same way, by setting  $C_1 = 0$  in equation (3)-(6), The linear strain relations are obtained:

$$\begin{Bmatrix} \varepsilon_{11} \\ \varepsilon_{22} \\ \varepsilon_{12} \end{Bmatrix} = \begin{Bmatrix} k_{11}^{(0)} \\ k_{22}^{(0)} \\ k_{12}^{(0)} \end{Bmatrix} + z \begin{Bmatrix} k^{(1)}_{11} \\ k^{(1)}_{22} \\ k^{(1)}_{12} \end{Bmatrix} \quad (35)$$

$$\begin{Bmatrix} \varepsilon_{13} \\ \varepsilon_{23} \end{Bmatrix} = \begin{Bmatrix} \gamma_{13}^{(0)} \\ \gamma_{23}^{(0)} \end{Bmatrix} \quad (36)$$

where:

$$\begin{cases} k_{11}^{(0)} \\ k_{22}^{(0)} \\ k_{12}^{(0)} \end{cases} = \begin{cases} \frac{\partial u_0}{\partial x} \\ \frac{\partial v_0}{\partial y} \\ \frac{\partial u_0}{\partial y} + \frac{\partial v_0}{\partial x} \end{cases}, \quad \begin{cases} k_{11}^{(1)} \\ k_{22}^{(1)} \\ k_{12}^{(1)} \end{cases} = \begin{cases} \frac{\partial \phi_1}{\partial x} \\ \frac{\partial \phi_2}{\partial y} \\ \frac{\partial \phi_1}{\partial y} + \frac{\partial \phi_2}{\partial x} \end{cases} \quad (37)$$

$$\begin{cases} \gamma_{13}^{(e)} \\ \gamma_{23}^{(e)} \end{cases} = \begin{cases} \left( \phi_1 + \frac{\partial w_0}{\partial x} \right) \\ \left( \phi_2 + \frac{\partial w_0}{\partial y} \right) \end{cases} \quad (38)$$

### 3.3. Equations of motion

The equation of motion of FSDT by using hamilton's principle are:

$$\frac{\partial N_{11}}{\partial x} + \frac{\partial N_{12}}{\partial y} = -q_1 + I_0 \ddot{u}_1 + I_1 \ddot{\phi}_1 \quad (39)$$

$$\frac{\partial N_{22}}{\partial y} + \frac{\partial N_{12}}{\partial x} = -q_2 + I_0 \ddot{u}_2 + I_1 \ddot{\phi}_2 \quad (40)$$

$$\frac{\partial Q_{13}}{\partial x} + \frac{\partial Q_{23}}{\partial y} = -q_3 + I_0 \ddot{u}_3 \quad (41)$$

$$\frac{\partial M_{11}}{\partial x} + \frac{\partial M_{12}}{\partial y} - Q_{13} = I_1 \ddot{u}_1 + I_2 \ddot{\phi}_1 \quad (42)$$

$$\frac{\partial M_{22}}{\partial y} + \frac{\partial M_{12}}{\partial x} - Q_{23} = I_1 \ddot{u}_2 + I_2 \ddot{\phi}_2 \quad (43)$$

where:

$$\begin{cases} \{N\} \\ \{M\} \end{cases} = \begin{bmatrix} [A] & [B] \\ [B] & [D] \end{bmatrix} \begin{cases} \{k^{(0)}\} \\ \{k^{(1)}\} \end{cases} \quad i, j = 1, 2, 6 \quad (44)$$

$$\{Q\} = [A] \{ \gamma^{(0)} \} \quad i, j = 4, 5 \quad (45)$$

and

$$(A_{ij}, B_{ij}, D_{ij}) = \int_{-h/2}^{h/2} Q_{ij}(1, z, z^2) dz \quad (46)$$

$$I_i = \int_{-h/2}^{h/2} \rho(z)^i dz \quad (47)$$

### 3.4. Natural frequency equation of simply supported FGM rectangular plate (FSDT)

Substitution of equations (25a)-(25e) into equations (39)-(43) and simplifying the resultants relations, then we obtain the following equation:

$$[C] \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \\ \phi_1 \\ \phi_2 \end{Bmatrix} - \omega^2 [M] \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \\ \phi_1 \\ \phi_2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix} \quad (48)$$

where:

$$C = \begin{bmatrix} c_{11} & c_{12} & c_{13} & c_{14} & c_{15} \\ c_{21} & c_{22} & c_{23} & c_{24} & c_{25} \\ c_{31} & c_{32} & c_{33} & c_{34} & c_{35} \\ c_{41} & c_{42} & c_{43} & c_{44} & c_{45} \\ c_{51} & c_{52} & c_{53} & c_{54} & c_{55} \end{bmatrix}, M = \begin{bmatrix} m_{11} & m_{12} & m_{13} & m_{14} & m_{15} \\ m_{21} & m_{22} & m_{23} & m_{24} & m_{25} \\ m_{31} & m_{32} & m_{33} & m_{34} & m_{35} \\ m_{41} & m_{42} & m_{43} & m_{44} & m_{45} \\ m_{51} & m_{52} & m_{53} & m_{54} & m_{55} \end{bmatrix} \quad (49)$$

The components of the matrix  $C$  are defined by:

$$\begin{aligned} c_{11} &= A_{11}\alpha^2 + A_{66}\beta^2 \\ c_{12} &= (A_{12} + A_{66})\alpha\beta \\ c_{13} &= 0 \\ c_{14} &= B_{11}\alpha^2 + B_{66}\beta^2 \\ c_{15} &= [B_{12} + B_{66}]\alpha\beta \\ c_{22} &= A_{66}\alpha^2 + A_{22}\beta^2 \\ c_{23} &= 0 \\ c_{24} &= c_{15} \\ c_{25} &= B_{66}\alpha^2 + B_{22}\beta^2 \\ c_{33} &= A_{55}\alpha^2 + A_{44}\beta^2 \\ c_{34} &= A_{55}\alpha \\ c_{35} &= A_{44}\beta \\ c_{44} &= A_{55} + D_{11}\alpha^2 + D_{66}\beta^2 \\ c_{45} &= [D_{12} + D_{66}]\alpha\beta \\ c_{55} &= A_{44} + D_{66}\alpha^2 + D_{22}\beta^2 \end{aligned} \quad (50)$$

And the components of the matrix  $M$  are defined by:

$$\begin{aligned} m_{11} &= I_0 \\ m_{22} &= I_0 \\ m_{33} &= I_0 \\ m_{34} &= 0 \\ m_{35} &= 0 \\ m_{44} &= I_2 \\ m_{55} &= I_2 \end{aligned} \quad (51)$$

The other components zero. To derive the natural frequency we set  $q=0$ , so (38)

becomes as following:

$$[C] \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \\ \phi_1 \\ \phi_2 \end{Bmatrix} - \bar{\omega}^2 [M] \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \\ \phi_1 \\ \phi_2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{Bmatrix}$$

or

$$[c_{ij} - m_{ij}\bar{\omega}^2] \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \\ \phi_1 \\ \phi_2 \end{Bmatrix} = 0 \quad (52)$$

Similar to TSDT, setting the determinant of matrix  $[c_{ij} - m_{ij}\bar{\omega}^2]$  equal to zero and solving the achieved equation for  $\bar{\omega}$  ( frequency), values of natural frequency for simply supported rectangular FGM plate will be derived.

#### 4. Result and discussion

The numerical results for an isotropic plate with  $\nu=0.3$  and FGM plate with  $\nu=0.3$ ,  $\rho_1/\rho_2=300/2707$  and  $G_1/G_2=151/70$  are given which  $\rho_1$  and  $\rho_2$  are ceramic and metal density. Also  $G_1$  and  $G_2$  are shear modulus of ceramic and metal, respectively. Also  $m,n$  are parameters mentioned in relations (25a)-(25e) and  $p$  relates to (8). The above results are the same for any FGMs according to (8).

From figures 1 to 10, we understand :

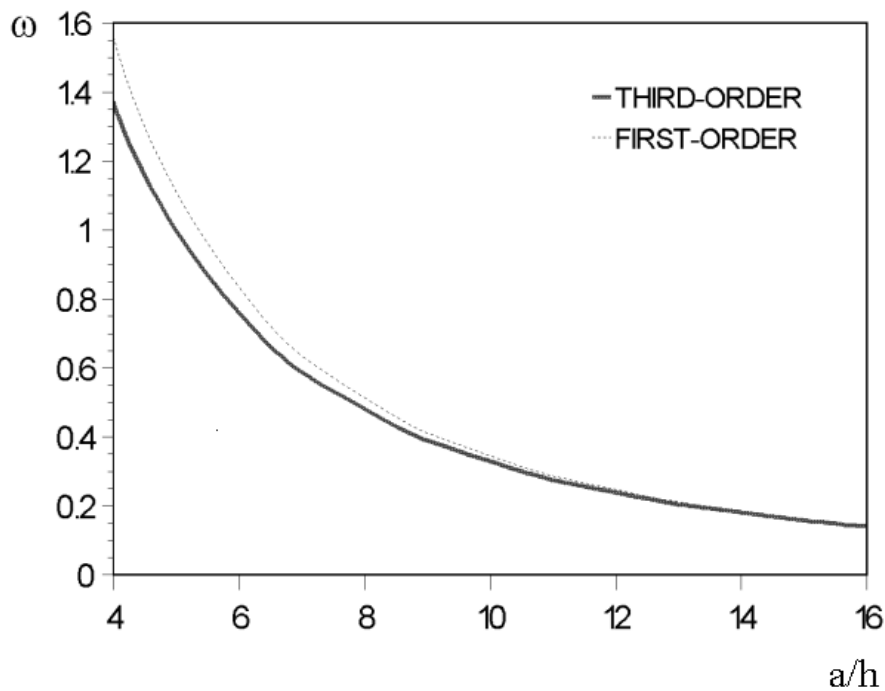
1. The nondimensionalized values of natural frequencies of both first and third theory decrease by increase of  $a/h$ , so that the difference between the nondimensionalized frequency in  $a/h=10$  and  $a/h=20$  is almost %80 but this decrease get shower and the figure becomes horizontal and the nondimensionalized natural frequencies with increase of  $a/h$ , leave constant.

2. By increasement of  $a/h$  the results of FSDT and HSDT are completely synchronize, in other words, by decrease of  $h$  (thickness), the results of FSDT and HSDT are completely synchronize. This matter is in derived relations, because  $h$  in HSDT effects on transvers shear stresses as a coefficient, but in FSDT the transverse shear stresses are constant along thickness and are independent of  $h$ . In other word with increase of  $h$  the difference between FSDT and HSDT increases, also the figures show that FSDT represent higher values.

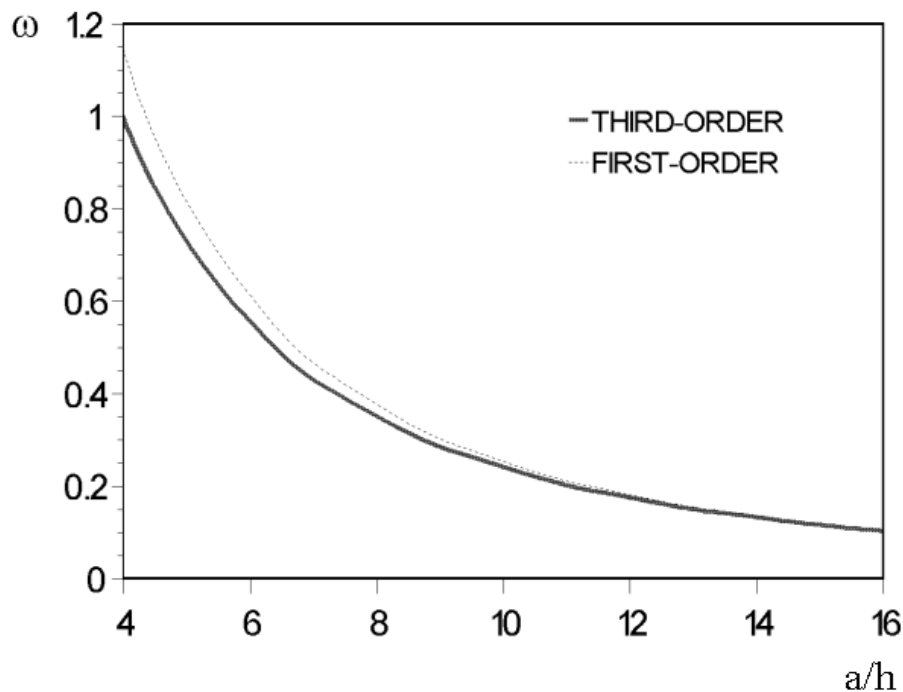
3. figure 11 shows that, with increase of  $p$ , the amount of natural frequency decrease, but at first the range of decrease is high and then get gradually and smoothly after. While  $p=0$  shows pure ceramic, so by increase of metal percentage the natural frequency decreases. As the elastisity modulus of ceramic is higher than elastisity modulus of metal, this fact is correct.

## 5. Conclusion

FSDT and HSDT theories can be used replace by another one for thin plates with high accuracy, but HSDT has higher accuracy for thick plates, so is better to use this theory for thick plates.

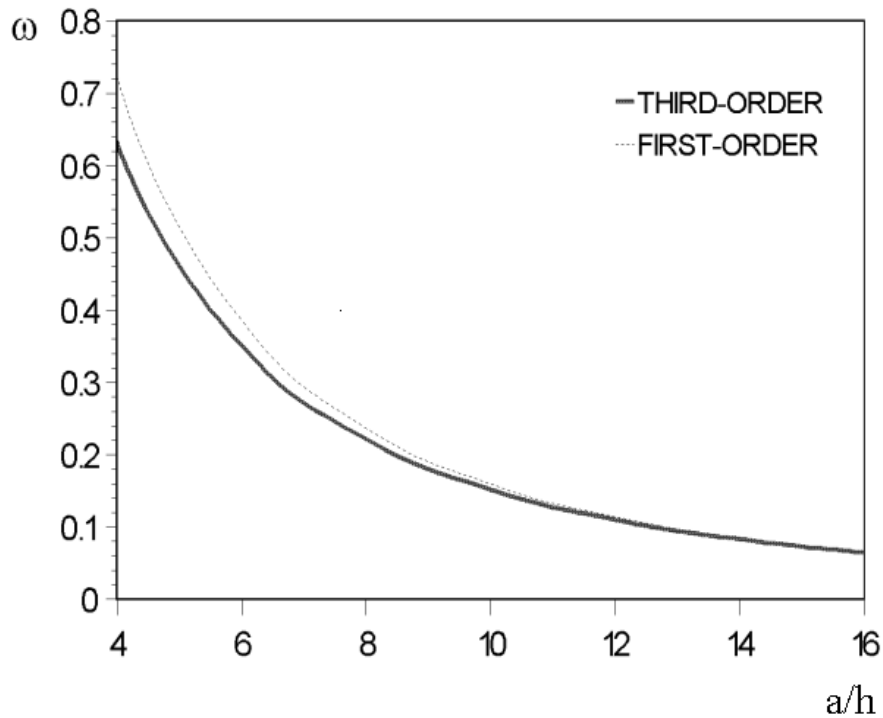


**Figure 1**-Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300 / 2707$ ,  $G_1 / G_2 = 151 / 70$  and  $p = 0$  (Isotropic-Material)

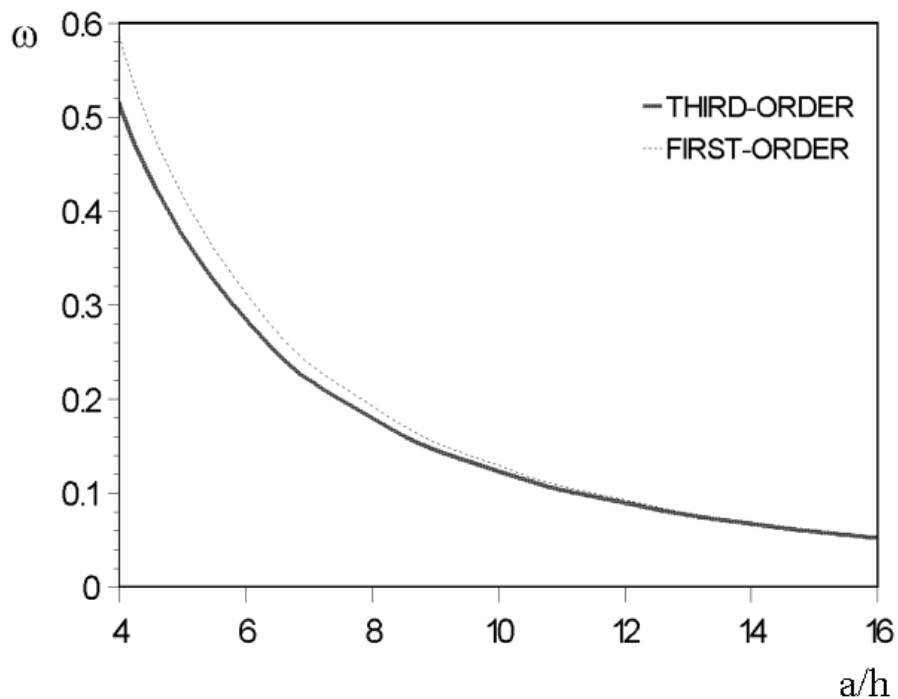


**Figure 2**-Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300 / 2707$ ,  $G_1 / G_2 = 151 / 70$  and  $p = 0.1$ .

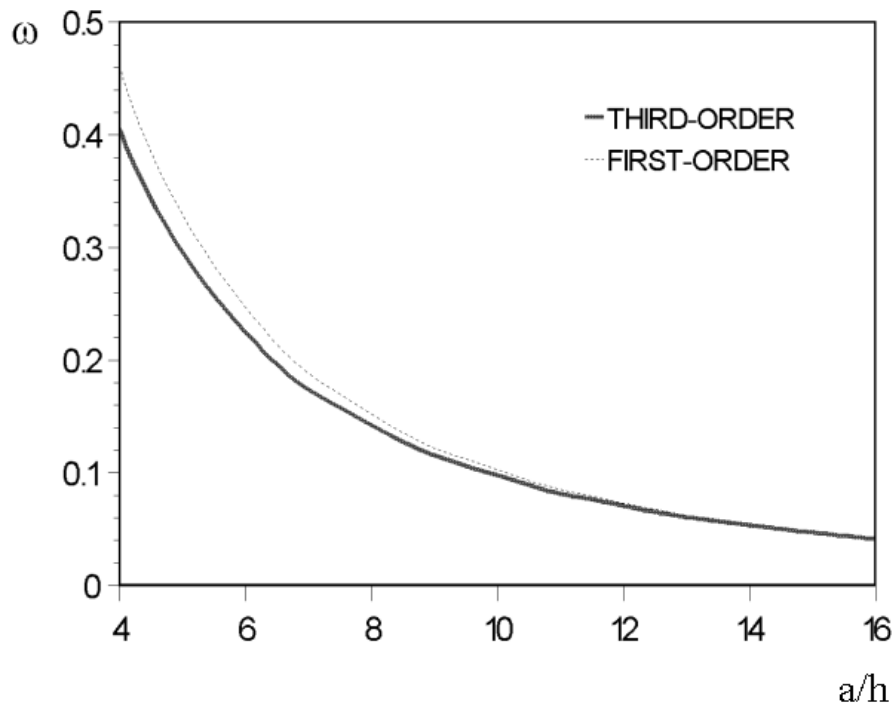




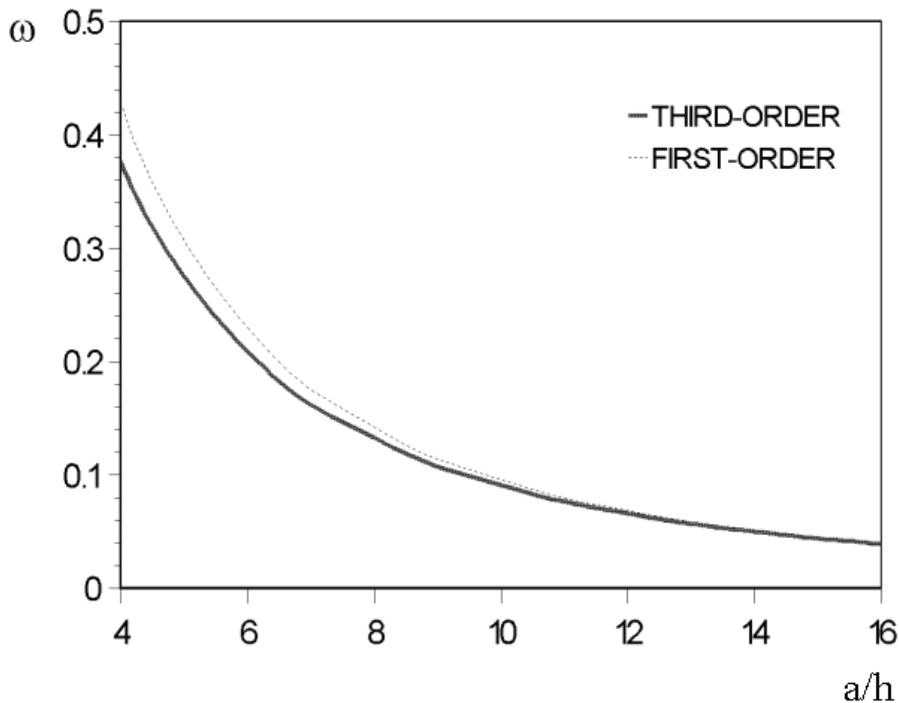
**Figure 3-**Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300 / 2707$ ,  $G_1 / G_2 = 151 / 70$  and  $p = 0.5$ .



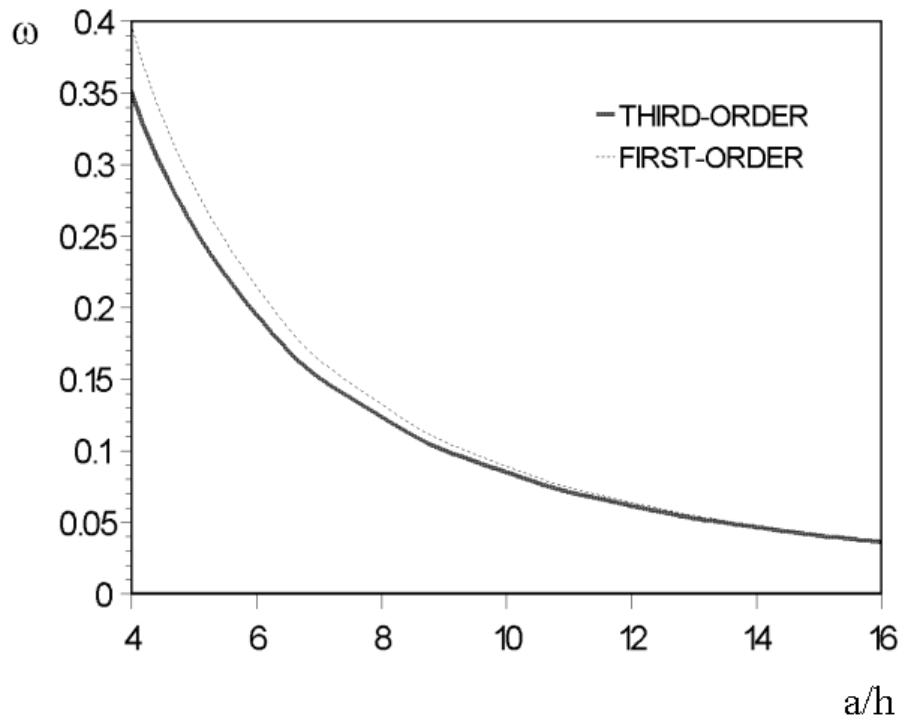
**Figure 4-**Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300 / 2707$ ,  $G_1 / G_2 = 151 / 70$  and  $p = 1$ .



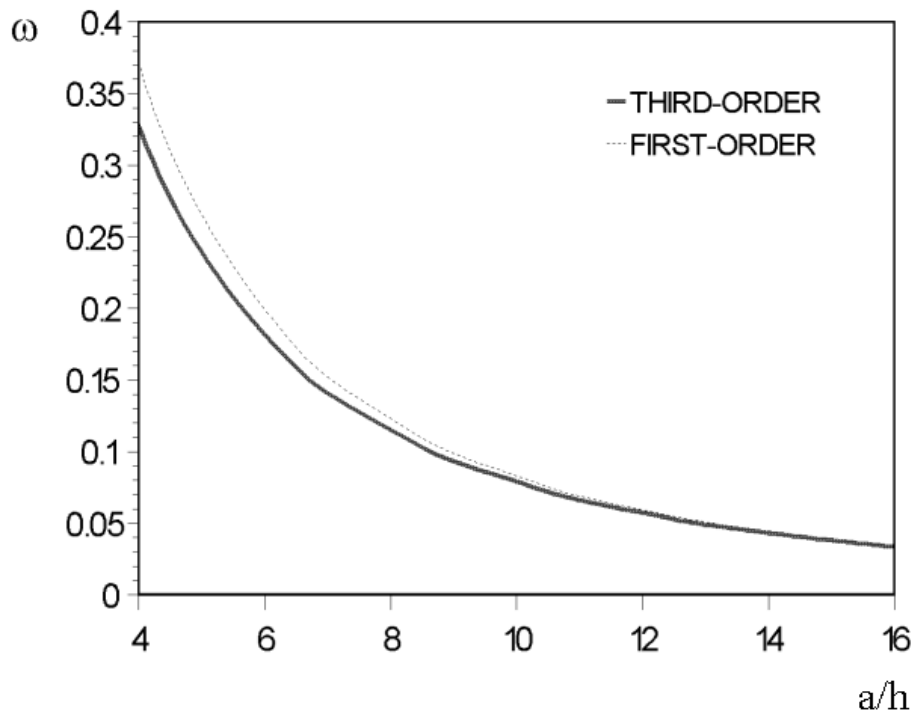
**Figure 5**-Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300 / 2707$ ,  $G_1 / G_2 = 151 / 70$  and  $p = 3$ .



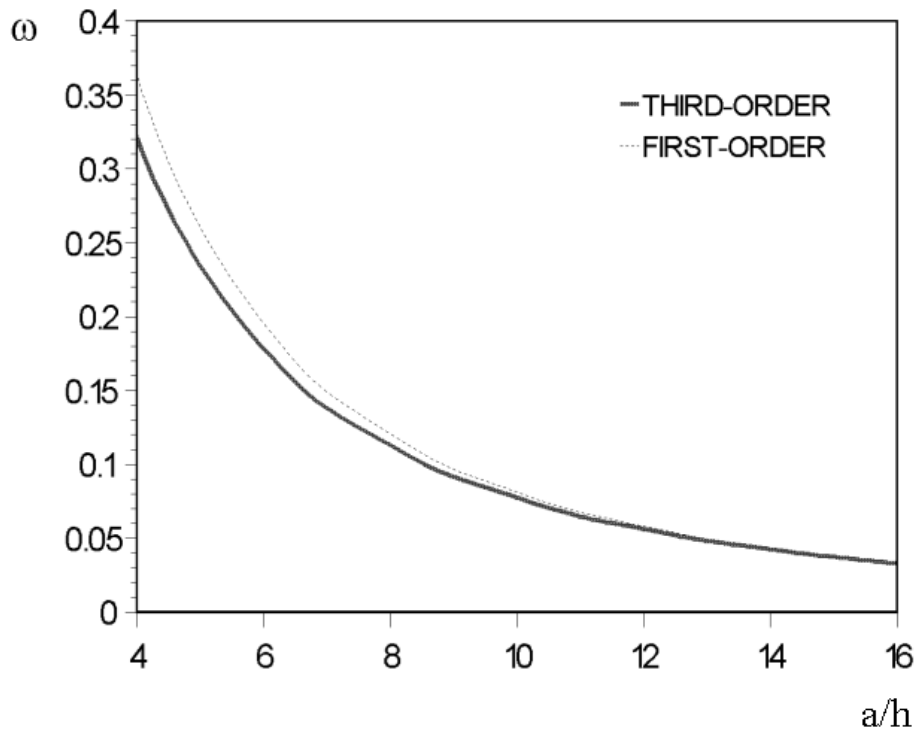
**Figure 6**-Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300 / 2707$ ,  $G_1 / G_2 = 151 / 70$  and  $p = 5$ .



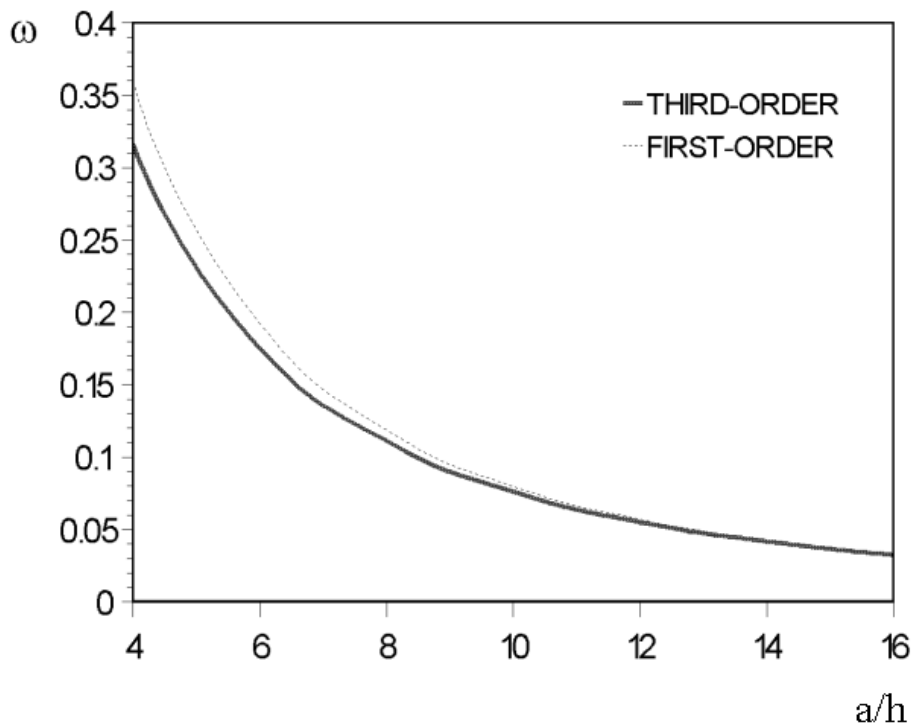
**Figure 7-**Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300/2707$ ,  $G_1 / G_2 = 151/70$  and  $p = 10$ .



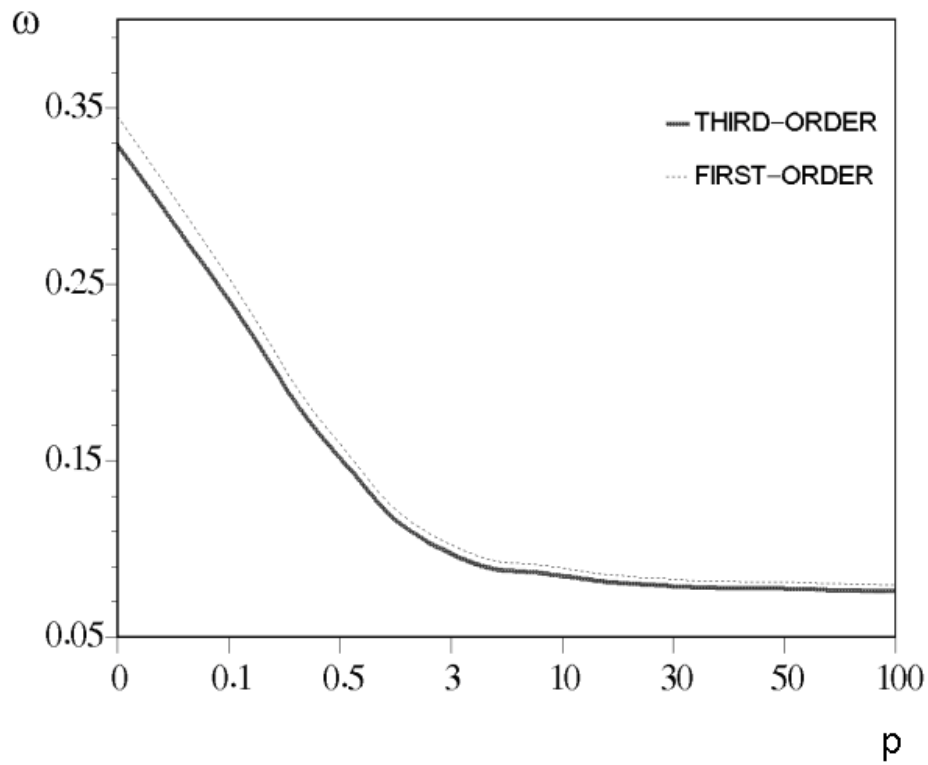
**Figure 8-**Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300/2707$ ,  $G_1 / G_2 = 151/70$  and  $p = 30$ .



**Figure 9-** Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300 / 2707$ ,  $G_1 / G_2 = 151 / 70$  and  $p = 50$ .



**Figure 10-** Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300 / 2707$ ,  $G_1 / G_2 = 151 / 70$  and  $p = 100$ .



**Figure 11-** Values of nondimensionalized natural frequency  $\bar{\omega} \cdot h \sqrt{\frac{\rho_1}{G_1}}$  for rectangular FGM plate in case  $m = n = 2$ ,  $\nu = 0.3$ ,  $\rho_1 / \rho_2 = 300 / 2707$ ,  $G_1 / G_2 = 151 / 70$ .

## References

- [1] M. Yamanouchi, M. Koizumi, I. Shiota, *Proceeding of the first International symposium on functionally gradient materials*, Sendai, Japan, 1990.
- [2] M. Koizumi, FGM activities in Japan, *Composite-Part B* 28 (1) (1997) 1-4.
- [3] J. N. Reddy, Z. Q. Cheng, Three-Dimensional transverse deformations of functionally graded rectangular plates, *European Journal of Mechanics A solid* 20 (2001) 841-855.
- [4] J. N. Reddy, C. M. Wang, S. Kitipornchi, Axisymmetric bending of functionally graded circular and annular plates, *European Journal of Mechanics A solid* 18 (1999) 185-199.
- [5] Y. Fukui, Fundamental investigation of functionally gradient material manufacturing system using centrifugal force, *International Journal of Japanese society of mechanical Engineering* 3 (1991) 144-148.
- [6] R. Javaheri, M. R. Eslami, Thermal buckling of functionally graded plates based on higher order theory, *Journal of Thermal Stresses* 25 (2003) 603-625.
- [7] J. N. Reddy, N. D. Phan, Stability and Vibration of isotropic, orthotropic and laminated plates according to a higher-order shear deformation Theory, *Journal of Sound and Vibration* 98 (2) (1985) 157-170.
- [8] J. N. Reddy, C. M. Wang, S. Kitipornchi, Relationship between vibration frequencies of Reddy and Kirchhoff polygonal plates with simply supported edges, *Journal of Vibration and Acoustics* 122 (2000) 77-81.
- [9] A. A. Khdeir, J. N. Reddy, Free vibration of laminated composite plate using second-order shear deformation theory, *Journal of Composite and Structures* 71 (1999) 617-626.
- [10] A. Nosier, R. K. Kapania, J. N. Reddy, Free vibration analysis of laminated plates using a Layerwise theory, *AIAA Journal* 31 (12) (1993) 2335-2345.
- [11] J. N. Reddy, N. D. Phan, Analysis of laminated composite plates using a higher order shear deformation theory, *International Journal for Numerical Methods in Engineering* 21 (1985) 2201-2219.
- [12] S. Timoshenko, K. Woinosky, *Theory of Plates and Shells* (second edition), McGraw-Hill, New York, 1959.
- [13] J. N. Reddy, A. A. Khdeir, Buckling and vibration of laminated composite plates using various plate theories, *AIAA Journal* 27 (12) (1989) 1808-2346.

- [14] J. N. Reddy, *Theory and Analysis of Elastic Plates*, Taylor & Francis, Philadelphia, PA, 1999.